

2.34, 2.38, 2.48, 2.64,

2.34.

Given: $P = 30 \text{ kN}$, $E_a = 70 \text{ GPa}$, $E_b = 105 \text{ GPa}$

Find: the normal stress

- (a) in the aluminum layers
- (b) in the brass layer.

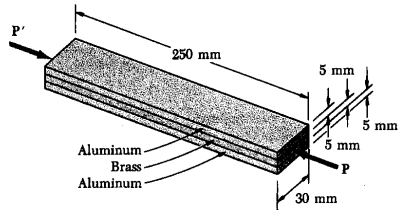
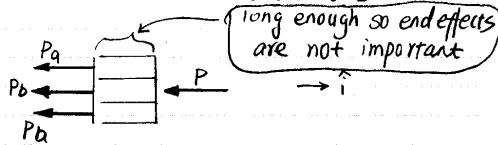


Fig. P2.34

Soln:

We need the axial force in each of the layer cut the whole and draw the F.B.D.



$$\sum F_x = 0: -P = 2P_a + P_b = -30 \text{ kN} \quad (1)$$

Need another condition to solve for forces:

$$\delta_a = \delta_b \quad (\text{they're attached to each other})$$

$$\delta_a = \frac{P_a L}{E_a A_a}$$

$$\delta_b = \frac{P_b L}{E_b A_b} \quad A_a = A_b$$

$$\Rightarrow \frac{P_a}{P_b} = \frac{E_b A_b}{E_a A_a} = \frac{E_b}{E_a} = \frac{70}{105} \quad (2)$$

Solving (1) & (2):

$$\text{from (2)} \Rightarrow P_a = 0.67 P_b \text{ put it to (1)} \Rightarrow$$

$$2 \times 0.67 P_b + P_b = -30 \text{ kN}$$

$$\Rightarrow \begin{aligned} P_a &= -8.6 \text{ kN} \\ P_b &= -12.8 \text{ kN} \end{aligned}$$

Cont. 2.34

$$\sigma_a = \frac{P_a}{A_a} = \frac{-8.6 \text{ kN}}{(5 \text{ mm})(30 \text{ mm})} = \frac{-8.6 \times 10^3 \text{ N}}{150 \times 10^{-6} \text{ m}^2} = -57.3 \text{ MPa}$$

$$\sigma_b = \frac{P_b}{A_b} = \frac{-12.8 \text{ kN}}{(5 \text{ mm})(30 \text{ mm})} = \frac{-12.8 \times 10^3 \text{ N}}{150 \times 10^{-6} \text{ m}^2} = -85.3 \text{ MPa}$$

$$\boxed{\begin{aligned} \sigma_a &= -57.3 \text{ MPa} \\ \sigma_b &= -85.3 \text{ MPa} \end{aligned}}$$

2.38

Given: $E_s = 200 \text{ GPa}$, $E_b = 105 \text{ GPa}$

- Find: (a) the reactions at A & E
- (b) the deflection of point C.

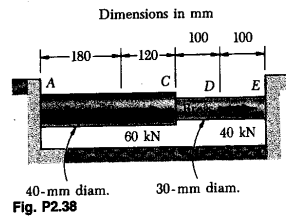
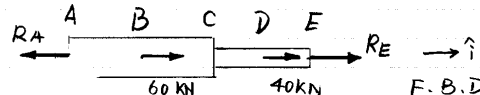


Fig. P2.38

Soln:

(a). We can get one eqn. from the force analysis.

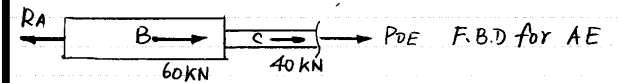
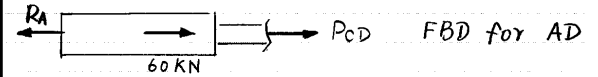
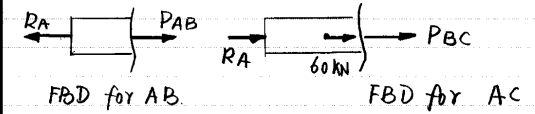


$$\sum F_x = 0: -R_A + 100 \text{ kN} + R_E = 0$$

Two unknowns, one eqn, we need the geometry condition.

$$0 = \delta_E = \frac{P_{AB} L_{AB}}{E_s A_s} + \frac{P_{BC} L_{BC}}{E_s A_s} + \frac{P_{CD} L_{CD}}{E_b A_b} + \frac{P_{DE} L_{DE}}{E_b A_b} = \delta_{AB} + \delta_{BC} + \delta_{CD} + \delta_{DE} \quad (1)$$

Need P_{AB} , P_{BC} , P_{CD} , P_{DE} : draw the F.B.D for each cutting:



From the above F.B.Ds, we have:

$$\begin{aligned} P_{AB} &= +R_A \\ P_{BC} &= -60 \text{ kN} + R_A \\ P_{CD} &= +R_A - 60 \text{ kN} \\ P_{DE} &= +R_A - 100 \text{ kN} \end{aligned}$$

plugging all these force back into (1) $\delta = 0 \Rightarrow$

$$\frac{(+R_A)(180 \text{ mm})}{E_s A_s} + \frac{(+R_A - 60 \text{ kN})(120 \text{ mm})}{E_s A_s} +$$

$$\frac{(+R_A - 60 \text{ kN})(100 \text{ mm})}{E_b A_b} + \frac{(+R_A - 100 \text{ kN})(100 \text{ mm})}{E_b A_b}$$

= 0

$$\Rightarrow \frac{1}{E_s A_s} [+R_A(0.3 \text{ m})] + \frac{1}{E_b A_b} [+R_A(0.2 \text{ m})] + \frac{1}{E_s A_s} [-7200 \text{ N}] + \frac{1}{E_b A_b} [-16000 \text{ N}] = 0$$

$$\Rightarrow R_A = +62.8 \text{ kN}$$

$$R_E = -R_A + 100 \text{ kN} = 37.2 \text{ kN}$$

$$\boxed{\begin{aligned} R_A &= -62.8 \text{ kN} \uparrow \\ R_E &= 37.2 \text{ kN} \uparrow \end{aligned}}$$

(b). Finding δ_c :

$$\delta_c = \frac{P_{AB} L_{AB}}{E_s A_s} + \frac{P_{BC} L_{BC}}{E_s A_s} = \frac{(62.8 \text{ kN})(180 \text{ mm}) + (+2.8 \text{ kN})(120 \text{ mm})}{(200 \text{ GPa})(\pi (20 \text{ mm})^2)}$$

$$= 46.3 \frac{\mu\text{m}}{(1 \mu\text{m} = 10^{-6} \text{ m})}$$

$$\boxed{\delta_c = 46.3 \mu\text{m}}$$

2.48:

Given: $E_A = 29 \times 10^6 \text{ psi}$ for link BC and DE, $\frac{1}{2}$ " wide, $\frac{1}{4}$ " thick, $P = 600 \text{ lb}$. AF rigid body.

Find: (a) Force in each link
(b) deflection A

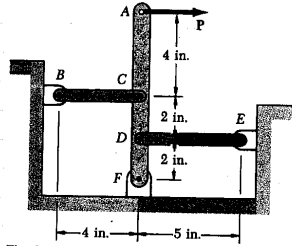
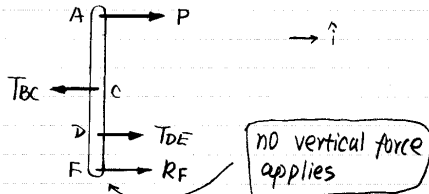


Fig. P2.48

Soln:

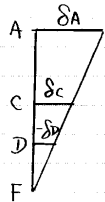
(a) Draw the FBD of AF:



$$\sum M_F = 0: -T_{DE}(2'') - P(8'') + T_{BC}(4'') = 0$$

$$\Rightarrow -T_{DE} + 2T_{BC} = 4P = 24 \text{ klb} \quad (1)$$

We have two unknowns, but one equation. Looking for geometry:



It's $-\delta_D$ here, because D will actually move to the left due to the tension in DE.

$$\frac{\delta_C}{-\delta_D} = \frac{4''}{2''} = 2 \Rightarrow$$

$$\frac{T_{BC} L_{BC}}{E_A A} = -2 \frac{T_{DE} L_{DE}}{E_A A} \Rightarrow$$

$$2T_{BC} + 5T_{DE} = 0 \quad (2)$$

Cont. 2.48.

Solving (1) and (2):

$$T_{DE} = -400 \text{ lb} \Rightarrow \begin{matrix} T_{DE} = -400 \text{ lb} \\ T_{BC} = 1000 \text{ lb} \end{matrix}$$

(b). Finding deflection in A:

From the geometry figure:

$$\frac{\delta_A}{\delta_C} = \frac{8''}{4''} = 2$$

$$\Rightarrow \delta_A = 2\delta_C = \frac{2T_{BC}L_{BC}}{E_A A}$$

$$= \frac{2(1000 \text{ lb})(4'')}{(29 \times 10^6 \text{ psi})(\frac{1}{2}'' \times \frac{1}{4}'')} = 2.2 \times 10^{-3} \text{ in}$$

$$\delta_A = 2.2 \times 10^{-3} \text{ in}$$

2.64.

Given: Rod AB w/ $E_b = 15 \times 10^6 \text{ psi}$, $\alpha_b = 11.6 \times 10^{-6} / ^\circ\text{F}$
Rod CD w/ $E_a = 10.1 \times 10^6 \text{ psi}$, $\alpha_a = 13.1 \times 10^{-6} / ^\circ\text{F}$
Gap of 0.02" is at 60°F

Find: (a) the temperature when $\sigma_{AB} = -20 \text{ ksi}$
(b) the deformation of AB At that time

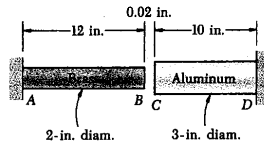


Fig. P2.63

Soln:

$$\delta_{AB} + \delta_{CD} = 0.02 \text{ in} \quad (1)$$

$$P_{AB} = P_{CD} \quad (\text{tension/compress in the rod})$$

$$\delta_{AB} = \frac{P_{AB}L_{AB}}{E_{AB}A_{AB}} + \alpha_{AB}\Delta T L_{AB} \quad (2)$$

Cont. 2.64

$$\delta_{CD} = \frac{P_{CD}L_{CD}}{E_{CD}A_{CD}} + \alpha_{CD}\Delta T L_{CD} \quad (3)$$

We also have:

$$\sigma_{AB} = \frac{P_{AB}}{A_{AB}}, \quad \sigma_{CD} = \frac{P_{CD}}{A_{CD}}$$

$$\Rightarrow P_{AB} = P_{CD} = \sigma_{AB} A_{AB} = P = (-20 \text{ ksi}) [\pi (1'')^2] = -62.83 \text{ kips}$$

\Rightarrow If we plug in (2) and (3) into (1), one eqn, one unknown which is ΔT .

$$0.02 \text{ in} = P \left(\frac{L_{AB}}{E_{AB}A_{AB}} + \frac{L_{CD}}{E_{CD}A_{CD}} \right) + \Delta T (\alpha_{AB}L_{AB} + \alpha_{CD}L_{CD})$$

$$\Rightarrow \Delta T = \frac{0.02 \text{ in} - P \left(\frac{L_{AB}}{E_{AB}A_{AB}} + \frac{L_{CD}}{E_{CD}A_{CD}} \right)}{\alpha_{AB}L_{AB} + \alpha_{CD}L_{CD}}$$

$$= 0.02'' - (-62.83 \text{ kips}) \left(\frac{12''}{(15 \times 10^6 \text{ psi}) \pi (1'')^2} + \frac{10''}{(10.1 \times 10^6 \text{ psi}) \pi (1.5'')^2} \right) + \frac{10''}{(11.6 \times 10^{-6} / ^\circ\text{F})(12'') + (13.1 \times 10^{-6} / ^\circ\text{F})(10'')}$$

$$= 165.8 \text{ } ^\circ\text{F}$$

$$\Rightarrow T = \Delta T + 60 \text{ } ^\circ\text{F} = 226 \text{ } ^\circ\text{F}$$

$$T = 226 \text{ } ^\circ\text{F}$$

(b). Finding δ_{AB}

$$\delta_{AB} = \delta_T^{AB} + \delta_P^{AB}$$

$$\delta_T^{AB} = \alpha_b \Delta T L_{AB} = (11.6 \times 10^{-6} / ^\circ\text{F})(165.8 \text{ } ^\circ\text{F})(12'') = 0.0231''$$

$$\delta_P^{AB} = \frac{P L_{AB}}{E_b A_b} = \frac{(-62.83 \text{ kips})(12'')}{(15 \times 10^6 \text{ psi}) \pi (1'')^2} = -0.016''$$

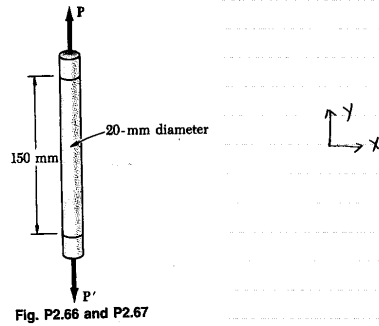
$$\Rightarrow \delta_{AB} = 0.007''$$

$$\delta_{AB} = 0.007''$$

2.66.

Given: $P = 30 \text{ kN}$, $E = 70 \text{ GPa}$, $\nu = 0.35$

Find: (a) δ of the rod
(b) the change in diameter of the rod.



Solution:

(a) Finding δ :

$$\delta = \frac{PL}{EA} = \frac{(30 \times 10^3 \text{ N})(150 \times 10^{-3} \text{ m})}{(70 \times 10^9 \text{ Pa}) \pi (10 \text{ mm})^2} = 0.205 \text{ mm}$$

$$\delta = 0.205 \text{ mm}$$

(b) Finding the change in diameter:

$$\epsilon_y = \frac{\delta}{L} = \frac{0.205 \text{ mm}}{150 \text{ mm}} = 1.36 \times 10^{-3}$$

$$\epsilon_x = -\nu \epsilon_y = -0.35 \times 1.36 \times 10^{-3} = -4.77 \times 10^{-4}$$

change in diameter $\Delta d = \epsilon_x d$:

$$\Delta d = -4.77 \times 10^{-4} \times 20 \text{ mm} = -9.54 \times 10^{-6} \text{ m} = -9.54 \mu\text{m}$$

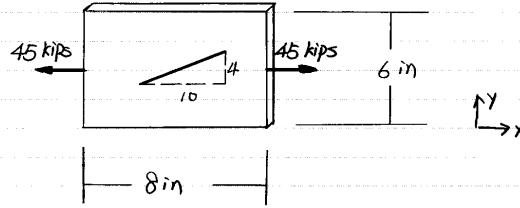
$$\Delta d = -9.54 \mu\text{m}$$

2.68.

Given: the data available in Appendix B for yellow-brass plate w/ $\frac{1}{4}$ " thick

Find: the slope of the line when $P = 45 \text{ kips}$.

Cont. 2.68.



Soln:

From Appendix B, for a yellow brass:

$$E = 15 \times 10^6 \text{ psi}, G = 5.6 \times 10^6 \text{ psi}$$

We have:

$$G = \frac{E}{2(1+\nu)} \Rightarrow$$

$$\nu = \frac{E}{2G} - 1 = \frac{15 \times 10^6 \text{ psi}}{2 \times 5.6 \times 10^6 \text{ psi}} - 1 = 0.3393$$

The slope of the line under $P = 45 \text{ kips}$ is:

$$\text{slope} = \frac{4(1+\epsilon_y)}{10(1+\epsilon_x)} \quad (\Delta L = \epsilon L)$$

We need to find ϵ_x, ϵ_y .

$$\sigma_x = \frac{P}{A} = \frac{45 \times 10^3 \text{ lb}}{\frac{1}{4} \text{ in} \times 6 \text{ in}} = 30 \times 10^3 \text{ psi}$$

$$\Rightarrow \epsilon_x = \frac{\sigma_x}{E} = \frac{30 \times 10^3 \text{ psi}}{15 \times 10^6 \text{ psi}} = 0.002$$

$$\epsilon_y = -\nu \epsilon_x = -0.3393 \times 0.002 = 6.8 \times 10^{-4}$$

$$\Rightarrow \text{slope} = \frac{4(1+6.8 \times 10^{-4})}{10(1+0.002)} = 0.39947$$

$$\text{slope} = 0.39947$$

2.86.

Given: Two blocks of rubber $G = 1.75 \text{ ksi}$ are bonded to rigid supports and plate AB.
 $C = 4$ ", $P = 10 \text{ kips}$

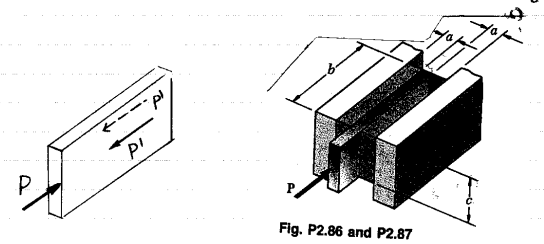
Find:

the smallest a and b if τ in the rubber $\leq 200 \text{ psi}$ and the deflection of the plate $\geq \frac{3}{16}$ ".

Cont 2.86.

Solution:

plate AB is under double shear from the geometry.



$$\Rightarrow P' = \frac{1}{2}P = 5 \text{ kips}$$

$$\tau = \frac{P'}{A} = \frac{5 \text{ kips}}{4 \text{ in} \times b} \leq 200 \text{ psi}$$

\Rightarrow

$$b \geq \frac{5 \times 10^3 \text{ lb}}{4 \text{ in} \times 200 \text{ psi}} = 6.25 \text{ in}$$

The deflection of the block = δ of the plate.

$$\delta = \gamma a = \frac{\tau}{G} a \geq \frac{3}{16} \text{ in}$$

$$\Rightarrow a \geq \frac{3 \text{ in}}{16} \frac{G}{\tau}$$

$$\geq \frac{3 \text{ in}}{16} \frac{1.75 \text{ ksi}}{200 \text{ psi}}$$

$$\geq 1.64 \text{ in}$$

$$a \geq 1.64 \text{ in}$$

$$b \geq 6.25 \text{ in}$$

*: Figure for δ for the block.

