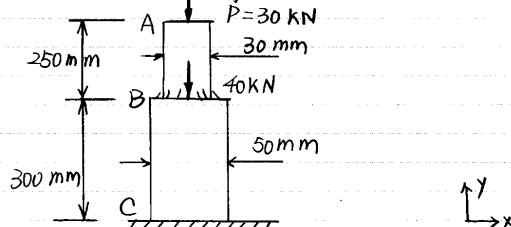


1.2. 1.20, 1.23, 1.36, 1.46, 2.2, 2.28

1.2. Two solid cylindrical rods are welded together at B. find the normal stress at the midpoint of each rod.



Soln:

Find the normal stress at the center of AB

$$\begin{aligned} \sum F_y &= 0, \\ T &= -P = -30 \text{ kN} \\ \Rightarrow O &= \frac{I}{A} = \frac{-30 \text{ kN}}{\pi (\frac{30}{2} \text{ mm})^2} \\ &= \frac{-30 \times 10^3 \text{ N}}{\pi 225 \times 10^{-6} \text{ m}^2} \\ &= -42 \text{ MPa} \end{aligned}$$

F.B.D. of the upper half AB

Assume stresses is uniform across cross section

For bar BC, draw the F.B.D. of the upper half BC plus AC:

$$\begin{aligned} \sum F_y &= 0: \\ -T &= P + 40 \text{ kN} = 70 \text{ kN} \\ \Rightarrow O &= \frac{I}{A} = -\frac{70 \times 10^3 \text{ N}}{\pi (\frac{50}{2} \text{ mm})^2} \\ &= -35.6 \text{ MPa} \end{aligned}$$

1.20 Find the normal stress in member AD, knowing the cross area of the member is 1200 mm²

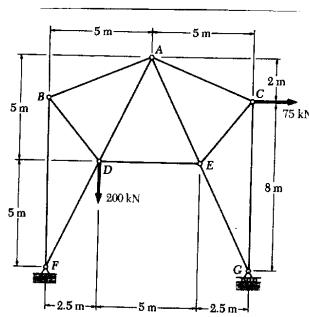
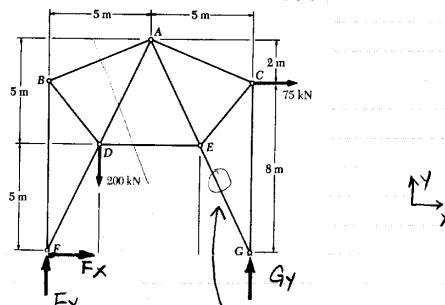


Fig. P1.19 and P1.20

Soln:

To find the normal stress in AD, we need to find the axial internal force in AD.

• Finding the reactions at F first



$$+\uparrow \sum M_H = 0 \Rightarrow$$

$$-F_y(10 \text{ m}) + 200 \text{ kN}(7.5 \text{ m}) - 75 \text{ kN}(8 \text{ m}) = 0$$

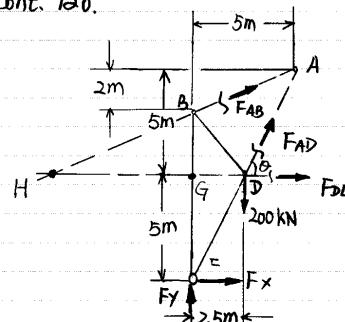
$$F_y = 90 \text{ kN}$$

$$(\sum \vec{F}) \cdot \hat{i} = 0$$

$$F_x = -75 \text{ kN}$$

• Cutting At AB, AD, DE and draw the FBD of BDF.

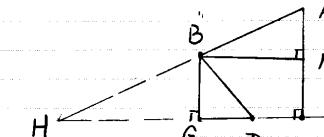
Cont. 120.



$$+\uparrow \sum M_H = 0 \Rightarrow$$

$$F_y \cdot (HG) - F_x(5 \text{ m}) + F_{AD}(d) - 200 \text{ kN}(HD) = 0$$

$d \rightarrow$ the distance from H to AD



"is similar to"
 $\triangle ABM \sim \triangle BHG$

$$\Rightarrow \frac{HG}{BM} = \frac{BG}{AM} = \frac{3 \text{ m}}{2 \text{ m}} = \frac{3}{2}$$

$$\Rightarrow HG = \frac{3}{2} \times 5 \text{ m} = 7.5 \text{ m}$$

$$\Rightarrow HD = HG + GD = 10 \text{ m}$$

$$\bullet F_{AD}(d) = F_{ADy} \cdot HD = F_{AD} \sin \theta \cdot 10 \text{ m}$$

$$\sin \theta = \frac{5}{\sqrt{5^2 + 2.5^2}} = 0.89$$

$$\Rightarrow +\uparrow \sum M_H = F_y \cdot (HG) + F_x(5 \text{ m}) + F_{AD} \sin \theta(10 \text{ m}) - 200 \text{ kN}(HD) = 90 \text{ kN}(7.5 \text{ m}) - 75 \text{ kN}(5 \text{ m}) + F_{AD} 0.89(10 \text{ m}) - 200 \text{ kN}(10 \text{ m})$$

$$\Rightarrow F_{AD} = \frac{(2000 + 375 - 675)}{0.89 \times 10 \text{ m}} = 191 \text{ kN}$$

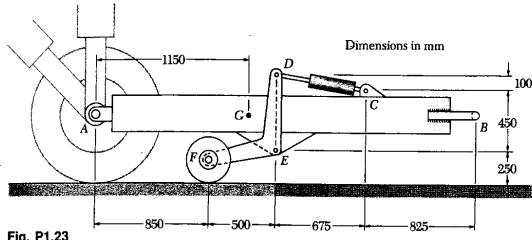
Cont. 1.20.

$$\sigma = \frac{F_{ad}}{A} = \frac{191 \text{ kN}}{1200 \text{ mm}^2} = \frac{191 \times 10^3 \text{ N}}{1200 \times 10^6 \text{ m}^2} = 159 \text{ MPa}$$

1.23.

Given: The diameter of steel rod = 25 mm
 DEF is a two identical arm and wheel units.
 The weight of the tow bar AB = 2 kN
 G is the center of mass of the tow

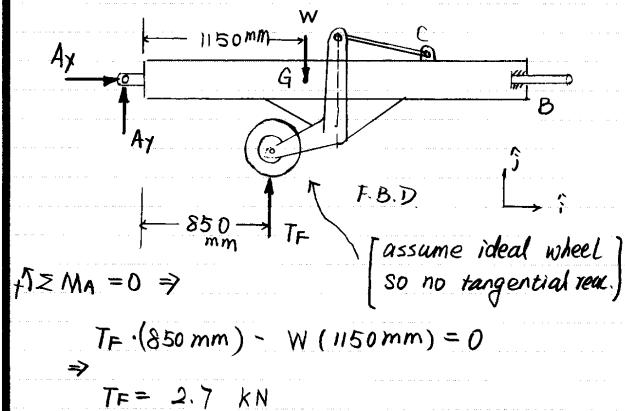
Find: The normal stress in the rod.



Solution:

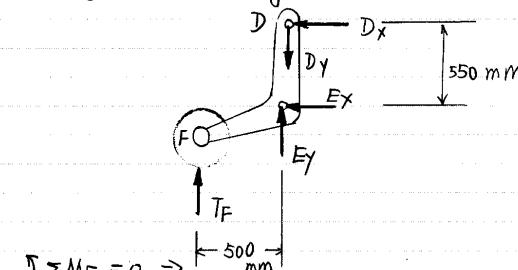
To find the normal stress in rod DC, we need to find the axial force in it.

• Study the bar and the units together at first.



Cont. 1.23

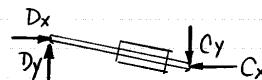
• Study the unit only



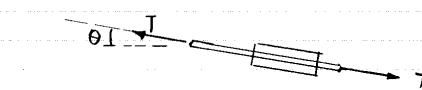
$$T_F(500 \text{ mm}) + D_x(550 \text{ mm}) = 0$$

$$\Rightarrow D_x = 2.45 \text{ kN}$$

• Draw the F.B.D. of the rod DC.



rod DC is a two force member.



$$\Rightarrow T \cos \theta = -D_x$$

$$\cos \theta = \frac{675 \text{ mm}}{\sqrt{675^2 + 100^2 \text{ mm}}} = 0.99$$

$$\Rightarrow T = \frac{-2.45 \text{ kN}}{0.99} = -2.47 \text{ kN} \quad (\text{compression})$$

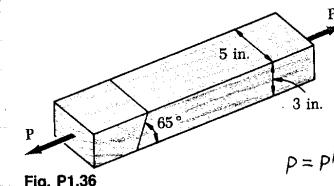
$$\Rightarrow \sigma = +\frac{T}{A} = -\frac{-2.47 \text{ kN}}{\pi \left(\frac{25 \text{ mm}}{2}\right)^2} = \frac{2.47 \times 10^3 \text{ N}}{\pi \cdot 156.25 \times 10^{-6} \text{ m}^2} = -5.03 \text{ MPa}$$

$$\sigma = -5.03 \text{ MPa}$$

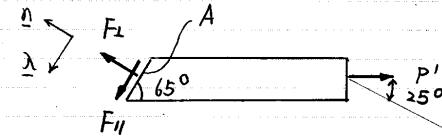
1.36

Given: 3x5-in uniform rectangular cross sect.
 for two members.
 $P = 800 \text{ lb}$

Find the normal and shearing stress in the glued joint.

 $P = P'$, obviously

Solution: Draw the F.B.D. of the right member



$$\{\sum F = 0\} \cdot n \Rightarrow$$

$$F_1 - P' \cos 25^\circ = 0$$

$$\Rightarrow F_1 = P' \cos 25^\circ = P \cos 25^\circ$$

$$\Rightarrow \sigma = \frac{F_1}{A} = \frac{P \cos 25^\circ}{A} = \frac{(800 \text{ lb}) \cos 25^\circ \sin 65^\circ}{3 \times 5 \text{ in}^2} = 43.8 \text{ psi}$$

$$\{\sum F = 0\} \cdot l \Rightarrow$$

$$F_{II} - P \sin 25^\circ = 0$$

$$\Rightarrow F_{II} = +P \sin 25^\circ$$

$$\tau = \frac{F_{II}}{A} = \frac{P \sin 25^\circ}{A \cdot l \sin 65^\circ} = \frac{(800 \text{ lb}) \sin 25^\circ \sin 65^\circ}{3 \times 5 \text{ in}^2} = 20.4 \text{ psi}$$

$$= 20.4 \text{ psi}$$

Cont. 1.36

$$\begin{aligned} T &= 20.4 \text{ PSI} \\ S &= 43.8 \text{ PSI} \end{aligned}$$

1.46. The wooden members are joined by plywood splice plates which are fully glued on the surface in contact. $\tau_{ult} = 2.5 \text{ MPa}$ in the glued joint.

Find: The factor of safety when $L = 180 \text{ mm}$

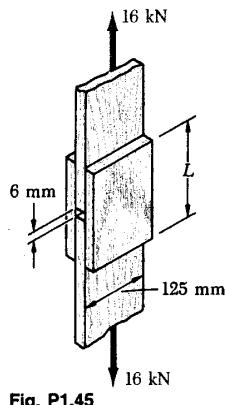


Fig. P1.45

V_1 is on the outer side
of the plate.
 V_2 is on the inner
side of the plate.

$$\text{Since: } F.S = \frac{\tau_{ult}}{\tau_{all}}$$

$$\text{we need to find } \tau_{all} = \frac{V}{A}$$

from the F.B.D of the upper plate: $\sum F_y = 0 \Rightarrow$

$$V_1 = V_2 = V = \frac{16 \text{ kN}}{2} = 8 \text{ kN}$$

$$\Rightarrow \tau_{all} = \frac{8 \text{ kN}}{A}$$

$$A = (125 \text{ mm}) (\frac{L - 6 \text{ mm}}{2}) = 10875 \text{ mm}^2$$

$$\Rightarrow F.S = \frac{\tau_{ult}}{\tau_{all}} = \frac{2.5 \text{ MPa}}{\frac{8 \times 10^3 \text{ N}}{10875 \times 10^{-6} \text{ m}^2}}$$

$$= \frac{2.5 \text{ MPa}}{0.736 \text{ MPa}} = 3.4 \quad F.S = 3.4$$

2.2

Given: when $T = 6 \text{ kN}$, $L = 60 \text{ m}$ for steel wire
 $\Delta L \leq 48 \text{ mm}$
 $E = 200 \text{ GPa}$

Find: (a) the smallest diameter for the wire
(b) the value of the normal stress

Solution:

(a).

$$\Delta L = \frac{PL}{EA} \text{ for axial loading}$$

$$\text{when } \Delta L = 48 \text{ mm} \Rightarrow$$

$$A = \frac{PL}{E\Delta L} \text{ will be smallest}$$

$$= \frac{(6 \text{ kN})(60 \text{ m})}{(200 \text{ GPa})(48 \times 10^{-3} \text{ m})}$$

$$= \frac{7.5 \times 10^6 \text{ N}}{200 \times 10^9 \text{ (N/m}^2\text{)}} \\ = 37.5 \text{ mm}^2$$

$$\Rightarrow d = 2\sqrt{\frac{A}{\pi}} = 2\sqrt{\frac{37.5 \text{ mm}^2}{\pi}} = 6.91 \text{ mm}$$

$$d = 6.91 \text{ mm}$$

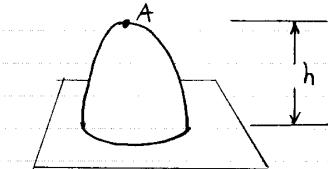
(b). when $d = 6.91 \text{ mm}$

$$\sigma = \frac{T}{A} = \frac{6 \text{ kN}}{37.5 \text{ mm}^2} = 160 \text{ MPa}$$

$$\sigma = 160 \text{ MPa}$$

2.28.

Determine the deflection of the apex A of the homogeneous paraboloid with height h , density ρ , due to its own weight.

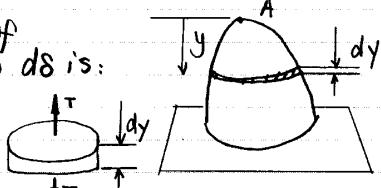


Cont. 2.28.

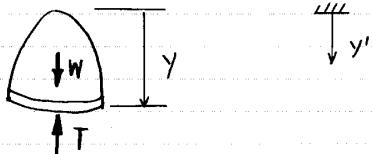
This's a problem regarding non-uniform A.

Take a small slab of thickness dy . \Rightarrow the deflection of this small slab $d\delta$ is:

$$d\delta = \frac{T dy}{EA}$$

because now the slab can be viewed as a circular plate with $\lambda = dy$

P will be the weight above the slab according to the F.B.D of the upper part of the slab.



$$V = \int_0^y A(y') dy'$$

$$= \int_0^y \pi r^2(y') dy'$$

$$= \pi \int_0^y [r(y')]^2 dy'$$

Eqn. of Volume for paraboloid from top to y.

For paraboloid, we have

$$r(y') = C\sqrt{y'} \quad C: \text{is for how fat the paraboloid is.}$$

$$\Rightarrow V = \pi \int_0^y (C\sqrt{y'})^2 dy'$$

$$= \pi C^2 \int_0^y y' dy'$$

$$= \frac{1}{2} \pi C^2 y^2$$

$$\Rightarrow T = -W = -89V = -89 \frac{1}{2} \pi C^2 y^2$$

$$\Rightarrow d\delta = \frac{T dy}{EA} = \frac{-89 \frac{1}{2} \pi C^2 y^2}{E \pi (Cy)^2} dy$$

Cont. 2.28.

where A at $y = \pi(C\sqrt{y})^2$, because now $y' = y$

$$\Rightarrow d\delta = -\frac{89y}{2E} dy$$

 \Rightarrow The deflection of point A is

$$\delta = \int d\delta = - \int_0^h \frac{89y}{2E} dy$$

$$= -\frac{89}{2E} \left(\frac{1}{2}y^2\right) \Big|_0^h$$

$$= -\frac{89}{4E} h^2$$

Note: C drops out! why? Two paraboloids side by side are like one fatter paraboloid.

$$\delta = -\frac{89h^2}{4E}$$

negative δ means the paraboloid gets shorter