

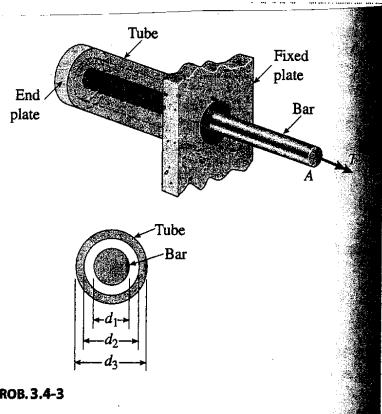
TAM202, HW 12 Solutions Prepared by Vijay Muralidharan
Due on Nov. 19, 2002

3.4-3 A circular tube of outer diameter $d_3 = 2.75$ in. and inner diameter $d_2 = 2.35$ in. is welded at the right-hand end to a fixed plate and at the left-hand end to a rigid end plate (see figure). A solid circular bar of diameter $d_1 = 1.60$ in. is inside of, and concentric with, the tube. The bar passes through a hole in the fixed plate and is welded to the end plate.

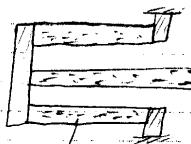
The bar is 40 in. long and the tube is half as long as the bar. A torque $T = 10,000$ lb-in. acts at end A of the bar. Also, both the bar and tube are made of an aluminum alloy with shear modulus of elasticity $G = 3.9 \times 10^6$ psi.

(a) Determine the maximum shear stresses in both the bar and tube.

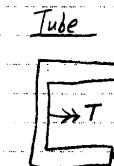
(b) Determine the angle of twist (in degrees) at end A of the bar.



PROB. 3.4-3

Solution'

Tube



Tube

$$d_3 = 2.75 \text{ in}, d_2 = 2.35 \text{ in}$$

$$G = 3.9 \times 10^6 \text{ psi}$$

$$L_{\text{tube}} = 20 \text{ in}$$

$$(J_P)_{\text{tube}} = \frac{\pi}{32} (d_3^4 - d_2^4)$$

$$= 2.621 \text{ in}^4$$

$$d_1 = 1.60 \text{ in.}$$

$$G = 3.9 \times 10^6 \text{ psi}$$

$$L_{\text{bar}} = 40 \text{ in}$$

$$(J_P)_{\text{bar}} = \frac{\pi}{32} d_1^4$$

$$= 0.6434 \text{ in}^4$$

(Continued)

Bar

Torque $T = 10,000$ lb-in

a) Maximum Shear Stresses

$$\text{Bar: } T_{\text{bar}} = \frac{16T}{\pi d_1^3}$$

$$= \frac{16 \times (10,000 \text{ lb-in})}{\pi (1.6 \text{ in})^3} = 12,430 \text{ psi}$$

$$\text{Tube: } T_{\text{tube}} = \frac{T(d_3/2)}{(J_P)_{\text{tube}}} = \frac{(10,000 \text{ lb-in})(2.75 \text{ in})}{2.621 \text{ in}^4}$$

$$= 5,250 \text{ psi}$$

$$b) \phi_A = \phi_{\text{bar}} + \phi_{\text{tube}}$$

$$\text{Bar: } \phi_{\text{bar}} = \frac{TL_{\text{bar}}}{G(J_P)_{\text{bar}}} = \frac{(10,000 \text{ lb-in})(40 \text{ in})}{(3.9 \times 10^6 \text{ psi})(0.6434 \text{ in}^4)}$$

$$= 0.1594 \text{ rad}$$

$$\text{Tube: } \phi_{\text{tube}} = \frac{TL_{\text{tube}}}{G(J_P)_{\text{tube}}} = \frac{(10,000 \text{ lb-in})(20 \text{ in})}{(3.9 \times 10^6 \text{ psi})(2.621 \text{ in}^4)}$$

$$= 0.0196 \text{ rad}$$

$$\phi_A = \phi_{\text{bar}} + \phi_{\text{tube}} = 0.1790 \text{ rad} = 10.3^\circ$$

$$[\phi_A = 10.3^\circ = 0.179 \text{ rad}]$$

3.4-10

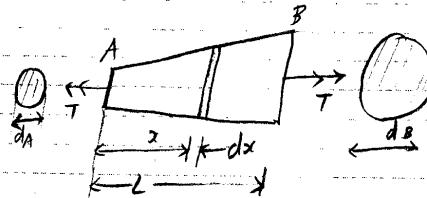


PROBS. 3.4-9 and 3.4-10

3.4-10 The bar shown in the figure is tapered linearly from end A to end B and has a solid circular cross section. The diameter at the smaller end of the bar is $d_A = 25$ mm. The bar is made of steel with shear modulus of elasticity $G = 82$ GPa.

If the torque $T = 90$ N-m and the allowable rate of twist is $0.5^\circ/\text{m}$, what is the minimum allowable diameter d_B at the larger end of the bar? (Hint: Use the results of Example 3-5.)

(Continued)



To determine the general expression for Angle of twist:

Diameter d at a distance x from end A,

$$d = d_A + \frac{d_B - d_A}{L} x \rightarrow ①$$

Polar moment of Inertia:

$$I_P(x) = \frac{\pi d^4}{32} = \frac{\pi}{32} \left[d_A + \frac{d_B - d_A}{L} x \right]^4$$

$$\therefore \phi = \int_0^L \frac{T dx}{G I_P(x)} \quad [\because \text{Torque } T \text{ is constant}]$$

$$\phi = \frac{32T}{\pi G} \int_0^L \frac{dx}{\left(d_A + \frac{d_B - d_A}{L} x \right)^4} \rightarrow ②$$

The above integral is of the form

$$\int \frac{dx}{(at^2x)^4} \quad \text{where } a = d_A, b = \frac{d_B - d_A}{L}$$

$$\text{Now, } \int \frac{dx}{(at^2x)^4} = -\frac{1}{3b(at^2x)^3} \rightarrow ③$$

∴ Using ③, ② gets simplified to:

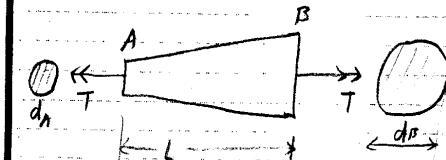
$$\phi = \frac{32 TL}{3\pi G(d_B - d_A)} \left(\frac{1}{d_A^3} - \frac{1}{d_B^3} \right) \rightarrow ④$$

A convenient form to ④ is

$$\phi = \frac{TL}{G(J_P)_A} \left(\frac{\beta^2 + \beta + 1}{3\beta^3} \right) \rightarrow ⑤$$

$$\text{where } \beta = \frac{d_B}{d_A}, (J_P)_A = \frac{\pi d_A^4}{32} \quad (\text{Continued})$$

Cont'd

Tapered Bar:

$$d_A = 25 \text{ mm}, G = 82 \text{ GPa}, T = 90 \text{ N}\cdot\text{m}$$

$$\theta_{\text{allow}} = 0.5' / \text{m}$$

Using ⑤, we get

$$\Theta = \frac{T}{G(J_p)_A} \left(\frac{\beta^2 + \beta + 1}{3\beta^3} \right)$$

$$\text{where, } \Theta = \frac{\phi}{L}, \beta = \frac{d_B}{d_A}, (J_p)_A = \frac{\pi}{32} d_A^4$$

$$\therefore 0.5'/\text{m} \left(\frac{\pi \text{ rad}}{180 \text{ deg}} \right) = \frac{90 \text{ N}\cdot\text{m}}{(82 \text{ GPa}) \left(\frac{\pi}{32} \right) (25 \text{ mm})^4} \left(\frac{\beta^2 + \beta + 1}{3\beta^3} \right)$$

$$\Rightarrow 0.3049 = \frac{\beta^2 + \beta + 1}{3\beta^3}$$

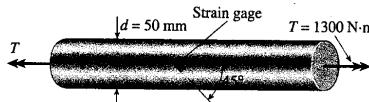
$$\text{Solving, } \beta = 1.9445$$

$$\therefore d_B = \beta d_A \Rightarrow d_B = 48.6 \text{ mm}$$

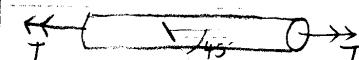
3.5-4

3.5-4 A solid circular bar of diameter $d = 50 \text{ mm}$ (see figure) is twisted in a testing machine until the applied torque reaches the value $T = 1300 \text{ N}\cdot\text{m}$. At this value of torque, a strain gage oriented at 45° to the axis of the bar gives a reading $\epsilon = 331 \times 10^{-6}$.

Determine the shear modulus G of the material.



PROB. 3.5-4



$$\text{Strain gage at } 45^\circ: \epsilon_{\text{max}} = 331 \times 10^{-6}$$

(continued)

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$$d = 50 \text{ mm}, T = 1300 \text{ N}\cdot\text{m}$$

From Eq. 3-33 (Gere),

$$Y_{\text{max}} = 2 E_{\text{max}} = 662 \times 10^{-6}$$

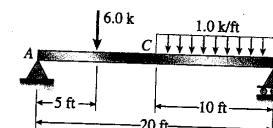
$$T_{\text{max}} = G Y_{\text{max}} = \frac{16 T}{\pi d^3}$$

$$\Rightarrow G = \frac{16 T}{\pi d^3 Y_{\text{max}}} = \frac{16 (1300 \text{ N}\cdot\text{m})}{\pi (0.05 \text{ m})^3 / 662 \times 10^{-6}}$$

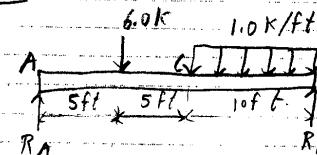
$$G = 80.0 \text{ GPa}$$

4.3-1

4.3-1 Determine the shear force V and bending moment M at the midpoint C of the simple beam AB shown in the figure.

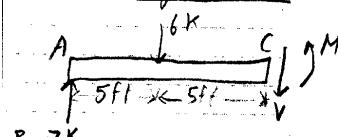


PROB. 4.3-1

Solution:

$$\sum M_B = 0: (6K \times 15ft) + (10K \times 5ft) - R_A \times 20ft = 0$$

$$\Rightarrow R_A = 7.0K$$

FBD of segment AC

$$R_B = 7K$$

$$\sum F_y = 0: V = 7K - 6K = 1.0K$$

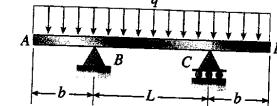
$$\sum M_C = 0: (6K \times 5ft) + M - (7K \times 10ft) = 0$$

$$\Rightarrow M = 40K \cdot ft$$

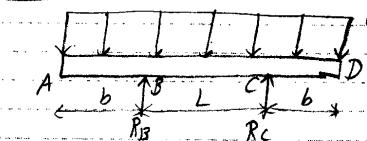
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4.3-4

4.3-4 The beam $ABCD$ shown in the figure has overhangs at each end and carries a uniform load of intensity q . For what ratio b/L will the bending moment at the midpoint of the beam be zero?

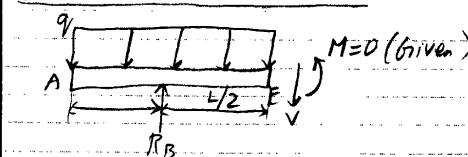


PROB. 4.3-4

Solution:

From symmetry and equilibrium in vertical direction,

$$R_B = R_C = \frac{q}{2} (L + 2b)$$

FBD of left half of beam:

$$\sum M_E = 0 (+):$$

$$-R_B \cdot \frac{L}{2} + q \cdot \frac{1}{2} \left(\frac{L}{2} + b \right)^2 = 0$$

$$\Rightarrow \frac{q}{2} \left(\frac{L}{2} + b \right)^2 = \frac{qL}{2} \left(\frac{L}{2} + b \right)$$

$$\Rightarrow \frac{L+b}{2} = L \Rightarrow b = L/2$$

$$\text{or } \frac{b}{L} = \frac{1}{2}$$

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